

# Standard Notes

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G. The contractor is expected to cross check all drainage inverts prior to commencing work, this may involve completion of trial holes if invert levels have been interpolated.

H. The contractor must monitor the "as build" progress of each construction stage (roads/sewers/plot works/to enable the next stages of construction to be checked before installation.

### <u>Highways</u>

All highway works to be carried out in accordance with the current local authority design guide and specification. 2. All excavations below proposed and existing highways to be back filled with granular

Type 1 sub base and well compacted in layers not exceeding 150mm, unless otherwise agreed. 3. Highway authority to be notified by the contractor prior to the commencement of

works. 4. All construction is to be undertaken in accordance with the current adopting authorities current standard details and under the supervision of the clerk of works. It is the responsibility of the contractor to familiarize themselves with these standards and organise inspections.

### Adoptable Drainage

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3. All brickwork to be Class B engineering complying with the relevant provisions of BS 3921. Concrete bricks maybe used if their specification is the same as Class B engineering bricks. Please seek approval from relevant water authority before using. 4. Manhole covers and frames shall comply with the relevant provisions of BS EN 124 and be of a non-rocking, non-ventilating design.

5. Ladders that are required in Type 1 manholes are to comply with "Sewers For Adoption 7th Edition".

6. Concrete must be either C20 sulphate resistant portland cement with high strength concrete topping to the benching or C35 ordinary portland cement. 7. 150mm Concrete surround is required around pipes where the depth from finished surface to soffit of pipe is less than 1200mm. This may be reduced to 900mm within open space.

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- 12. New gully positions and connections.

13. Position and depth of service ducts for water, gas, electric, BT, cable and street lighting, stating size and number of ducts.

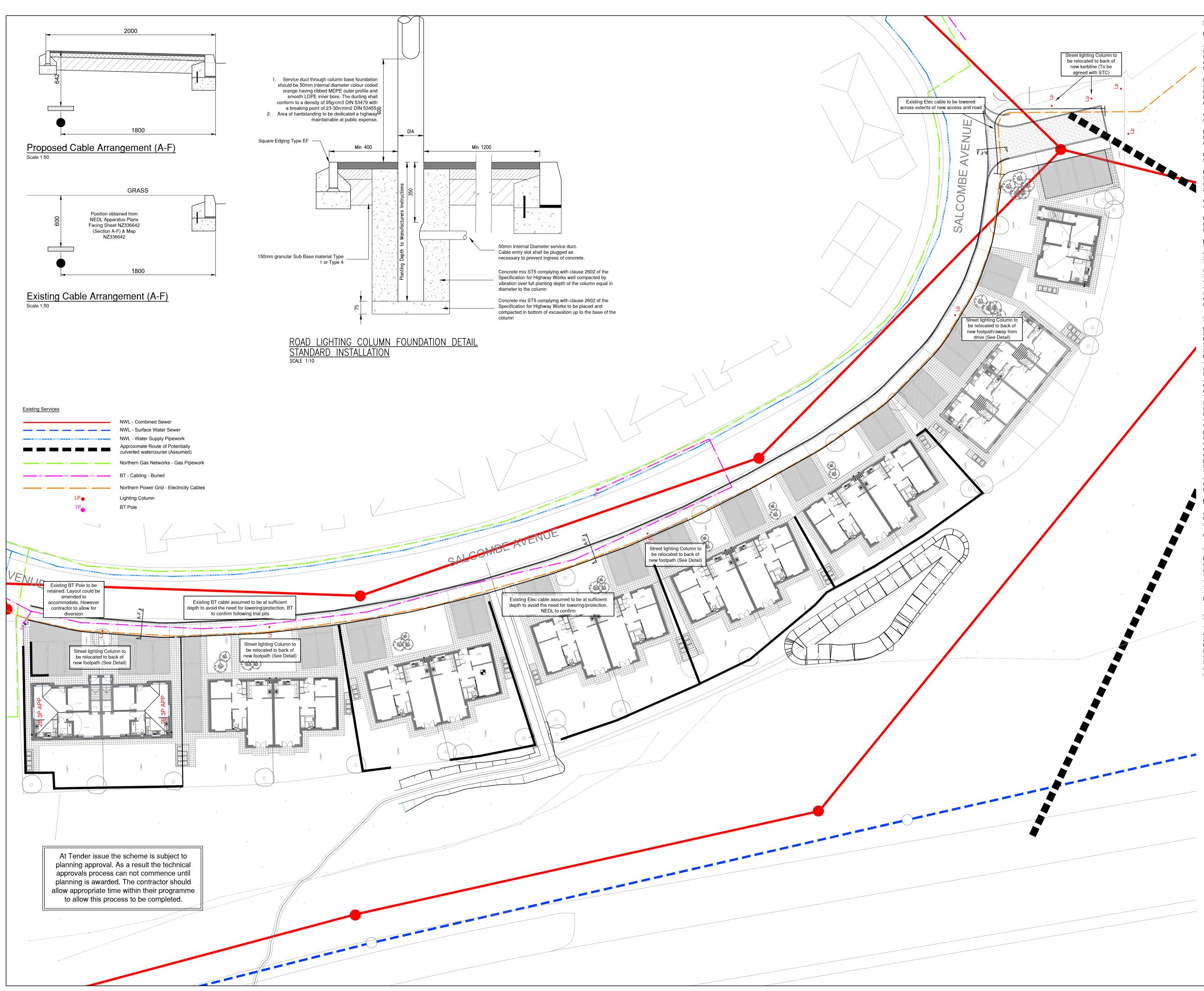
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3. Any amendments requested on site by the clerk of works, following technical approval of the drawings, resulting in additional works should be verified by the engineer and client or clients representative prior to commencement.

T1	T1 030217 Issued for Tender								
Rev	Date		Description		Drawn	Chkd			
Proje	Project SALCOMBE AVENUE, JARROW RESIDENTIAL DEVELOPMENT								
Client	Client South Tyneside								
Archit	Architect								
Title	Title EXISTING SERVICES								
Scal	e 1:250		Drawn DW		Date DEC	C 16			
Job	Number		Drawing Number	Rev.					
	1613	4	C-GA-0	T1					
www	CK21 Ltd. Shakespeare House, 18 Shakespeare St, Newcastle upon Tyne, NE1 6AQ, www.ck21.co.uk initial.surname@ck21.co.uk Telephone: (0191) 261 6312								
Stat	status TEIERDER								





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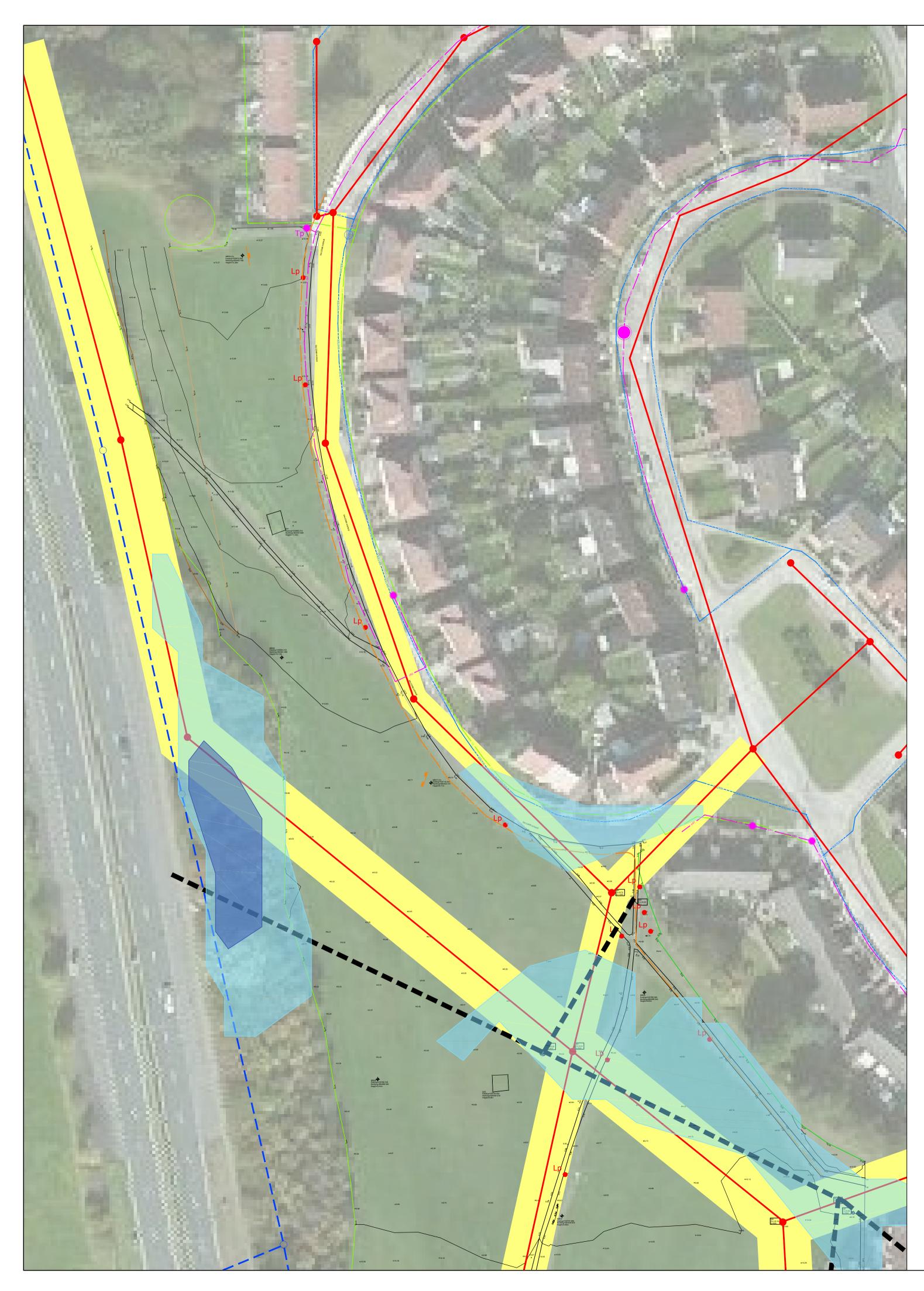
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Rev	Date	Description	KS Drawn	DW Chkd				
Proje	Project SALCOMBE AVENUE, JARROW RESIDENTIAL DEVELOPMENT							
Client	Client South Tyneside							
Archit	Architect CEAD							
Title	Title PROPOSED SERVICE DIVERSIONS							
Scal	e 1:250	Drawn DW	Date DE(	C 16				
Job	Number	Drawing Number	Rev.					
	1613	4 C-GA-002	Т	1				
	CK21 Ltd. Shakespeare House, 18 Shakespeare St, Newcastle upon Tyne, NE1 6AQ, Telephone: (0191) 261 6312							
-	www.ck21.co.uk initial.surname@ck21.co.uk Telephone: (0191) 261 6312 Status TENDER							





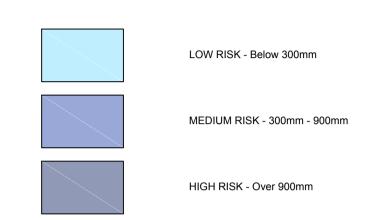
### Site Address Salcombe Avenue, jarrow, South Tyneside Nearest Postcode NE32 3SN Easting 433683 Northing 564372 Nat Grid NZ336643 Legend :

The topographical information contained is based on the survey undertaken by CENTARA on behalf of DUNELM.

Existing Services

	NWL - Combined Sewer NWL - Surface Water Sewer
WATER WATER WATER	NWL - Water Supply Pipework Approximate Route of Potentially culverted watercourse (Assumed)
EG EG EG	Northern Gas Networks - Gas Pipework
EBT EBT	BT - Cabling - Buried
E E E	Northern Power Grid - Electricity Cables

### Environment Agency - Flood Risk from surface water:



# Trees



## Geological & Geophysical Features

# Overview

The below are extracts from the DUNELM Geotechnical & Environmental Phase 1 Desktop Study, Report No. D7716

### Geology

The site is shown to be underlain by drift deposits comprising Pelaw Clay deposits. Glacial sands and gravels are present in the northwest corner of the site. In addition, the geological plan indicates an area of made ground through the centre of the site, in the area of the former stream/sewage pipe, indicated as Made ground/disturbed ground may also be encountered in the area of the embankments. The solid geology underlying the site comprises Coal Measures strata. The strata dip towards the southeast. No coal seams are shown to outcrop in the vicinity of the site, however, due to the faulting on site and in the surrounding area, the shallowest coal seam beneath the site is considered to be an unnamed coal seam, up to 0.9m thick, recorded as banded.

An un-named fault is shown through the north of the site, with a downthrow to the north. No significant ground hazards have been identified by the British Geological Survey as reported in the Groundsure Report.

### Mining & Quarrying

The mining report indicates that the site may be underlain by workings in two coal seams, the shallowest being the Maudlin Seam at a depth of approximately 332m below ground level and a thickness of 0.58m.

At this depth and thickness, working in this seam are not considered to pose a significant risk to the development. Based on the geological maps, the site is situated above the Wear Mouth Marine Band. Therefore, the shallowest coal seam beneath the site would be a thin unnamed coal seam and an unnamed seam up to 0.9m thick. Based on the vertical section, the coal would be at a depth of >15m below rock head.

Therefore, given the thickness of the seam at 0.9m, sufficient rock cover should be present to mitigate against workings in this seam, should they be present. There are no recorded mine entries within 20m of the site.

No evidence has been found to suggest that the site has been affected by quarrying.

### Landfills & Other Potential Gas Sources

The Groundsure Report indicates two recorded landfill sites located within 250m of the site, the closest being approximately 102m west of the site at NGR 433400, 563700. Whilst operational, authorized wastes included industrial, commercial and household waste. The landfill was active from 1950 to 1973.

Historical plans indicate that a pond 30m to the southwest of the site was backfilled prior to 1965. Consequently there may be made ground at this location together with organic sediments that could represent a potential source of gas. In addition, the historical plans and geological plans indicate an area of made ground through the centre of the site, possibly associated with a former stream/drain present in this The nature of the infill is unknown, however this feature could represent a potential source of gas.

Radon Gas In accordance with the procedure described in BRE Publication BR211 Radon: Guidance on Protective Measures for New Dwellings, no radon protection measures are required for new buildings on the site.

_T1	KS	DW						
Rev	Date	Description	Drawn	Chkd				
Proje	Project PORLOCK, JARROW RESIDENTIAL DEVELOPMENT							
Client	Client South Tyneside							
Archit	Architect							
Title ENGINEERING CONSTRAINTS PLAN								
Scal	e 1:500	Drawn DW	Date DE	C 16				
Job	Number	Drawing Number	Rev.					
	1612	4 C-GA-005	T	1				
	CK21 Ltd. Shakespeare House, 18 Shakespeare St, Newcastle upon Tyne, NE1 6AQ, www.ck21.co.uk initial.surname@ck21.co.uk Telephone: (0191) 261 6312							
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### Discharge Rates

- 1. The pre development site is a greenfield site, measuring 0.38ha, with no obvious natural surface water outfall. the existing topography falls North/South and East/West. Existing overland flows are likely to run east into the wooded area and be blocked by the embankment of the adjacent A19.
- 2. Consideration of the surface water hierarchy has been undertaken. Review of the Site Investigation report, produced by Dunelm, Dec 16, confirms that the underlying strata is not suitable to accept infiltration.
- The closest natural watercourse is approx 300m to the north of the site, which considering the topography and existing built environment, is considered an unacceptable distance/level to connect to directly.
- A Pre development enquiry has been submitted to NWL, applying for consent to discharge into the surface water sewer that runs SOUTH/NORTH within the western boundary of the site.
- 3. It is proposed that the surface water run off generated by the proposed development will be attenuated on site and released to the receiving watercourse at equivalent greenfield runoff rates. The existing greenfield run off rates from the development area (excluding the non contributing soft landscaping areas) have been calculated as follows:

Methodology (IH124): QBar = 0.95 l/s 1 in 1 year = 0.82 l/s 1 in 30 year = 1.67 l/s 1 in 100 year = 1.98 l/s

- In accordance with best practice a minimum flow rate of 5 l/s will be applies to any site Greenfield runoff calculations have been provided separately
- 4. It is proposed to utilize SuDS in combination with a suitable flow control device to restrict and attenuate flows from site to the above rates. A Bio-Retention basin has been selected as an appropriate method of SuDS to treat the proposed flows. Bioretention systems are shallow landscaped areas that can reduce runoff rates and volumes, and treat pollution through the use of engineered soils and vegetation. They are particularly effective in delivering interception and can also provide;
- attractive landscape features that are self irrigating and fertilizing habitat and biodiversity.
- 5. The system including the SuDS has been designed to accommodate the 100yr 360min storm event, without flooding, whilst ensuring that any flood volume from the critical 100 year event or as a result of system failure remains within the development boundary.
- 6. The SuDS Feature(s) will be maintained by the client (or management company) to ensure the performance of the drainage system is maintained throughout the design life of the development. A maintenance schedule will be provided separately.
- 7. The actual (at planning stage) calculated discharge rates into the receiving watercourse are as follows.

### 1 in 1 year = 4.2 l/s 1 in 30 year = 4.5 l/s = 4.5 l/s 1 in 100 year

These rates are derived from the Windes software model and include an allowance for 40% climate change. The windes calculations have been provided separately.

At Tender issue the scheme is subject to planning approval. As a result the technical approvals process can not commence until planning is awarded. The contractor should allow appropriate time within their programme to allow this process to be completed.

At time of issue the NWL Pre Development Enquiry response was still awaited. Drainage design may be subject to change following NWL comment/approval

At time of issue the Planning Approval was not received. Technical approval of the drainage scheme by the LLFA can only occur post planning. As a result the Drainage design may be subject to change pending STC LLFA comment/approval

### Treatment

# pollution mitigation index (for each containment type) the pollution hazard index (for each contai

		Table 1			$\bigcirc$
Land Use	Pollution Hazard Leve	Total Suspended Solids (TSS)	Metals	Hydrocarbons	$\bigcirc$
Residential Roofs	Very Low	0.2000	0.2000	0.0500	$\bigcirc$
Individual Property Driveways, residential car parks, low traffic roads	Very Low	0.5000	0.4000	0.4000	_

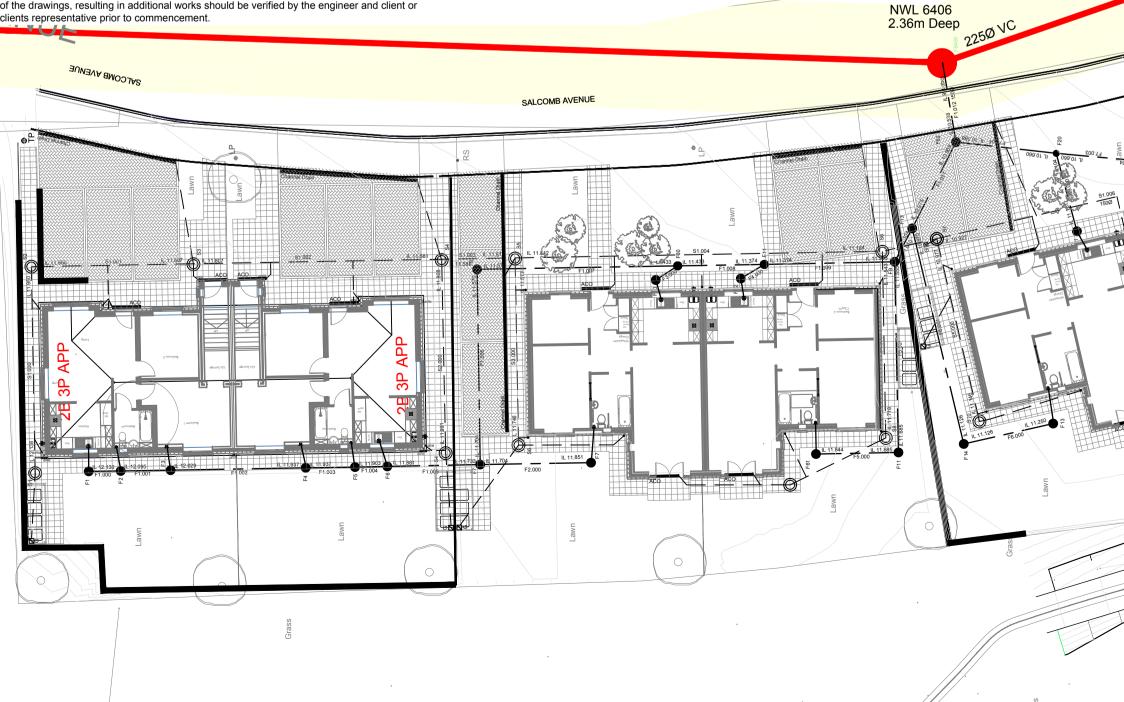
# Types of SUDS Component Bio-retention Basi

Table 2 - based on Table 26.3 - Indica surface waters

## Conclusion Reference to the above criteria confirm

provides sufficient SUDS mitigation fro sources in compliance with the requirements of CIRIA C753, The SUDS Manual (2016), section 26.7

Manhole Number	Cover Level				Pipe		Manhole Size	T	ypes	Manhole Number			Pipe			Manhole Size	Ту	Types	
Coordinates	Depth To Soffit	Connections		Code	Inverts	Diams Inv-soff		Manhole	Cover	Coordinates	Depth To Soffit	Connections		Code	Inverts	Diams Inv-soff		Manhole	Cover
S17	9.500		1	1.015	7.017	225			675x675	F1	9.500		1	F18.004	8.452	150			675x675
E. 433665.523 N. 564331.003	4.000 ASSUMED TBC	0	0	1.016	5.5 (tbc)	225	1200	A	D400	E. 433750.99 N. 564315.81	ASSUMED		0	EXISTING	твс	375	1500	A	D400
S16	9.110	2	1	1.014 12.006	8.400 7.256	225 150												-	
E. 433684.161	1.754			12.000	1.200	100	1200	В	675x675 D400		FOUL	. WATEF		IANHO	LE 3	CHE	DULE		
E. 433684.161 N. 564335.222		↓ 0	0	1.015	7.131	225													
014		1	1	1.012	8.751	225													
S14	9.615								4040-075										
E. 433693.780	0.639						1350	E	1240x675 D400										
N. 564341.283		V 0	0	1.013	8.751	225													
S12	10.430	$\wedge$	1 2	10.001 1.010	8.970 9.533	225 225									/		202	,C	
E 422706 275	1.360	2					1350	E	1240x675 D400							NWL 73 2.25m [	303 Deep	2250 10	
E. 433706.375 N. 564347.967		↓ 0	0	1.011	8.845	225										ALCOMB AVE	NUE		
S19	9.400	1	1	12.005	7.694	150									S	ALCO		10	
E 433704 645	1.556	$\bigcirc$					1200	В	675x675 D400									alle all	
E. 433704.645 N. 564319.711		↓ 0	0	12.006	7.694	150										A REAL PROPERTY AND		Lawr	



# Reference to the CIRIA C753, The SUDS Manual (2015), section 26.7, details the method to determine the SUDS pollution mitigation indices.

# To deliver adequate treatment, the selected SUDS components should have a total s or exceeds the

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ainment type)	

annic	ni type)	
ution	hazard	index

Existing NWL Combined Sewer Existing NWL Surface Water Sewer ----- Proposed Private SW Drainage - Proposed Private FW Drainage SW/FW Inspection Chambers (450Ø) SW/FW PCC Chambers (≥1200Ø) Flow Control Device Gully Road Gully Existing NWL Sewer Easement

Drawing Specific Notes

Legend

1. General levels prepared for planning submission. Levels may be subject to some minor alteration during design development.

2. Topographic Survey shown in background in feint.

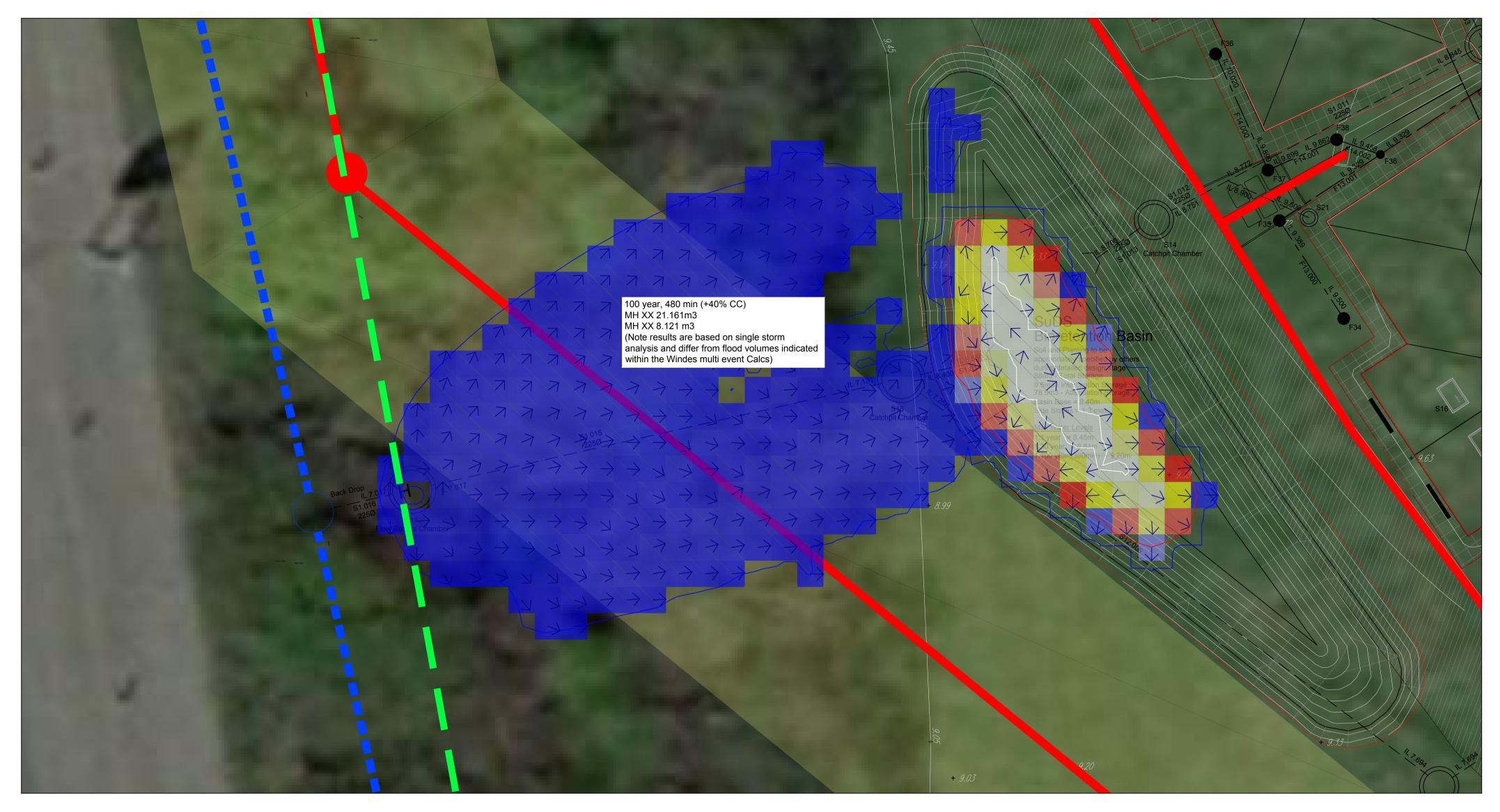
ative SuDS mitigation indices for discharge	e to

rms that the BIO-RETENTION BASIN alone	
rom the new build residential scheme. Treating all	

# SURFACE WATER MANHOLE SCHEDULE

NWL 6301 5.90m Deep





Post Development - Critical Event - Flood Flow Analysis - 100yr 480min + 40% climate change DO NOT USE THIS DRAWING IN ISOLATION

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G. The contractor is expected to cross check all drainage inverts prior to commencing work, this may involve completion of trial holes if invert levels have been interpolated.

H. The contractor must monitor the "as build" progress of each construction stage (roads/sewers/plot works/to enable the next stages of construction to be checked before installation.

Highways 1. All highway works to be carried out in accordance with the current local authority design guide and specification. All excavations below proposed and existing highways to be back filled with granular 2

Type 1 sub base and well compacted in layers not exceeding 150mm, unless otherwise agree 3. Highway authority to be notified by the contractor prior to the commencement of

works.

Adoptable Drainage 1. All adoptable drainage works to be in accordance with the water authorities publication "Sewers For Adoption 7th Edition" aswell as the approved drawings. 2. Precast concrete manhole rings to comply with the relevant provisions of BS5911:

Part 200. 3. All brickwork to be Class B engineering complying with the relevant provisions of BS 3921. Concrete bricks maybe used if their specification is the same as Class B engineering bricks. Please seek approval from relevant water authority before using. 4. Manhole covers and frames shall comply with the relevant provisions of BS EN 124

and be of a non-rocking, non-ventilating design. 5. Ladders that are required in Type 1 manholes are to comply with "Sewers For Adoption 7th Edition".

6. Concrete must be either C20 sulphate resistant portland cement with high strength concrete topping to the benching or C35 ordinary portland cement. 7. 150mm Concrete surround is required around pipes where the depth from finished surface to soffit of pipe is less than 1200mm. This may be reduced to 900mm within open space.

8. The location of existing drainage that is within close proximity to the proposed site works, which is not to be diverted, should be confirmed by the contractor and reported to the developer to ensure it corresponds to that shown on the engineering layout and that no proposed works are affected. The position, line and diameter of all existing drainage apparatus should be confirmed on site prior to the commencement of the works. Any discrepancies must be reported to the engineer immediately. The connection of foul and surface water drainage to the existing public sewer system shall be subject to the approval of the local sewerage undertaker. The contractor is expected to apply for relevant permits

prior to commencing the work. 9. Roads and sewers contractor must inform water authority prior to works commencing

## Existing Services

Any existing services which may be affected by the proposed works should be located by means of a hand dig in close liaison with the statutory service authorities. The contractor shall inform the developer of any services that may affect the proposed design.

Contractor to notify statutory service authorities prior to commencement of work.

As Constructed Information

Refer to note H above. It is the contractors responsibility to provide the following as constructed drawings to the developer upon the completion of the works covered by the

- Position/coordinates of all adoptable manholes. 2. Invert and cover levels of all adoptable manholes.
- New gully positions and connections. З.
- 4. Position and depth of service ducts for water, gas, electric, BT, cable and street lighting, stating size and number of ducts.

- Drawing Specific Notes
- 1. General levels prepared for planning submission. Levels may be

subject to some minor alteration during design development. 2. Topographic Survey shown in background in feint.

		T1	030217	Issue	ed for Tender		KS	DW		
Legend		Rev	Date		Description		Drawn	Chkd		
	Development Boundary	Project SALCOMBE AVENUE, JARROW RESIDENTIAL DEVELOPMENT								
	Existing NWL Combined Sewer									
	Existing NWL Surface Water Sewer									
	Proposed Private SW Drainage	Client South Tyneside								
	Proposed Private FW Drainage									
$\bigcirc$ •	SW/FW Inspection Chambers (450Ø)	Archit	ect							
$\bigcirc$ $\bullet$	SW/FW PCC Chambers (≥1200Ø)		C							
Н	Flow Control Device					\ \				
G	Gully		FLOOD FLOW ANALYSIS CRITICAL EVENT (100yr 480min)							
RG	Road Gully									
	Existing NWL Sewer Easement	Scale 1:200			Drawn DW		Date DEC 16			
		Job Number			Drawing Number		Rev.			
Flood Flow			1612	24	C-GA-2	211	T	1		
77	Depth 10 - 100mm				VOI	CK21 Ltd. Shakespe				
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7+9.33	Depth 301 - 600mm	State					/	0012		

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